DESIGN REPORT

Sedimentation Structure

TPC-A

Kayenta Mine

Navajo County, Arizona

for

PEABODY COAL COMPANY



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INTRODUCTION

Sedimentation Structure TPC-A will be a concrete wall with a small earthen wing embankment, designed and constructed by Peabody Coal Company as a temporary sedimentation structure to control runoff and sediment from the disturbed mining areas of the Kayenta Mine. The location of Structure TPC-A is shown on Plate 1, Site Plan.

This design report contains information specific to Structure TPC-A. Regional site information is presented in the "General Report, Kayenta and Black Mesa Mines, Navajo County, Arizona for Peabody Coal Company," along with the methods and results of analyses used for slope stability, hydrology and hydraulics.

INSPECTION

The proposed site of Structure TPC-A was inspected by a senior geotechnical engineer from Dames & Moore in October, 1985 to ensure that the site is suitable and no adverse conditions exist to prevent the successful construction of the structure. A detailed geotechnical investigation was not performed.

SITE DESCRIPTION

LAND USE

Structure TPC-A has a 2.5-acre tributary drainage area and is located near Long House Valley at the Kayenta Mine. The watershed is classified as 100% disturbed.

CONCRETE DAM

A vertical concrete wall with a notch spillway was assumed for the hydraulic analysis and to develop the volume-elevation curve shown on Plate 2.

DESIGN ANALYSES

GENERAL

Structure TPC-A was designed by an interdisciplinary team of engineers from Dames & Moore. The design was performed in accordance with applicable 30 CFR 780 and 816 regulations of the United States Department of Interior, Office of Surface Mining (OSM) and included a review of available project files. The most current information contained in the Peabody Coal Company files includes topographic maps developed from aerial photography flown in 1982 for Peabody Coal Company and was used in the analyses of the structure.

STABILITY

The concrete wall of Structure TPC-A was designed based on current ACI codes for concrete structures to be stable. The section shown on Plate 4 was checked for static stability (overturning and sliding), cantilever strength during overflow and seismic stability. The design assumes that the wall is founded in fresh competent sandstone bedrock.

HYDROLOGY

The hydrologic analysis was completed using the U.S. Army Corps of Engineers generalized computer program HEC-1, Flood Hydrograph Package. Structure TPC-A is not in series with any other structure and therefore the spillway was analyzed using the 25-year, 6-hour storm. The storage capacity of Structure TPC-A was analyzed using the 10-year, 24-hour storm.

The following parameters were used in the hydrologic analysis:

0.0625 mi Elevation Difference, H 25 2. Time of Concentration, T 0.031 3. 0.019 Lag time, 0.6T SCS Curve Number 94 Rainfall Depth, 10-year, 24-hour storm. 2.1 in. 25-year, 6-hour storm. . 1.9 in. 7. acres

HYDRAULICS

The HEC-1 program was used to evaluate inflow to the planned sedimentation structure, outflow from the structure and the resulting water surface elevations. The initial conditions and results of the analysis are summarized in the following table.

TPC-A HYDRAULICS

Units	10-year 24-hour Storm	25-year 6-hour Storm
Initial Reservoir Volume		
Condition	Empty	Full to the spillway elevation
Inflow		
Peak Flow cfs	8	10
Volume acre-ft	0.31	0.26
Storage		
Peak Stage ft	7308.39	7309.40
Spillway Elevation ft	7309.20	
Peak Storage acre-ft	0.31	
Storage Capacity acre-ft	0.48	
Outflow		
Peak Flow cfs Embankment Crest	0	9
Elevation ft		7310.50
Peak Stage ft		7309.40
Freeboard ft		1.10
Spillway Channel		
Flow Depth ft		0.20
Critical Velocity fps		2.1
Manning's "n"		0.040

Spillway Channel

The spillway for TPC-A will be a rectangular notch in the concrete dam.

The alignment of the spillway is shown on Plate 1. The profile is shown on Plate 3 and the required wall dimensions and details are shown on Plate 4.

STORAGE CAPACITY

The impoundment volume-elevation curve shown on Plate 2, Volume-Elevation Curve, TPC-A is based on site specific topographic data developed for Peabody Coal Company in 1985, and 1985 site specific surveys, where available.

The calculations for the sediment load entering Structure TPC-A were made utilizing the Universal Soil Loss Equation with the following parameters:

- 1. Rainfall Factor, R 40
- 2. Soil Erodibility Factor, K 0.14
- Slope Factor, LS 1.68
- 4. Cover Factor, C 1.0
- 5. Erosion Control Factor, P 1.0

The hydrologic analysis gives the storage volume required to contain the 10-year, 24-hour storm, and the remaining storage volume available for storing sediment. The storage capacity of TPC-A is shown on Plate 2, Volume-Elevation Curve, TPC-A, and the results of the sediment inflow analysis are summarized in the following table.

TPC-A STORAGE

Total Storage Capacity 0.48 acre-ft 10-year, 24-hour Storm Inflow 0.31 acre-ft Available Sediment Storage Capacity . 0.17 acre-ft Sediment Inflow Rate 0.011 acre-ft/yr Sediment Storage Life 15 yrs

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The following plates and appendix are attached and complete this design report.

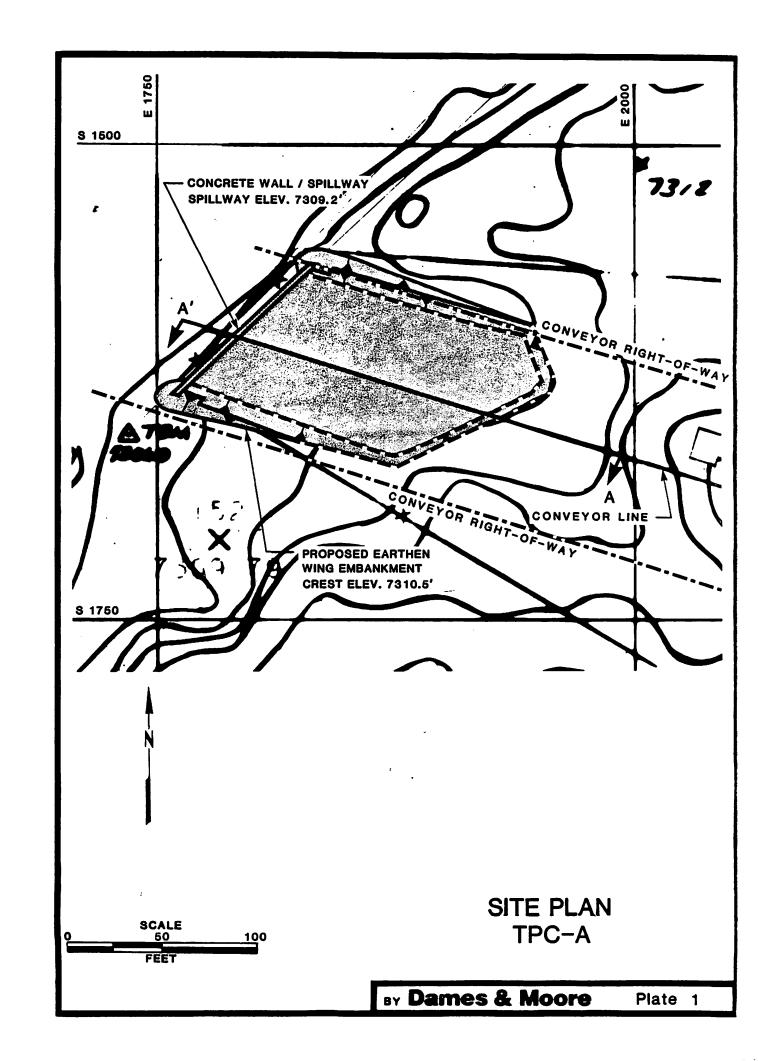
Plate 1 - Site Plan TPC-A

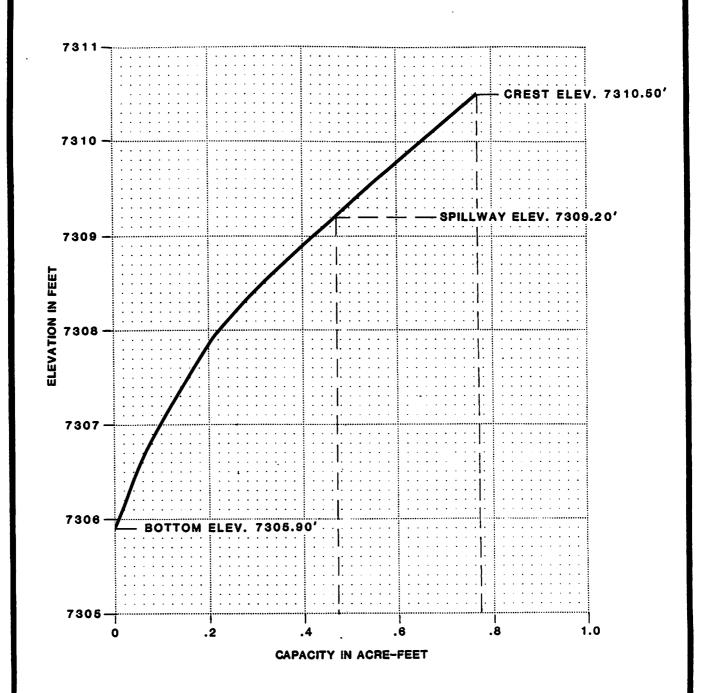
Plate 2 - Volume-Elevation Curve TPC-A

Plate 3 - Channel Profile TPC-A, A-A'

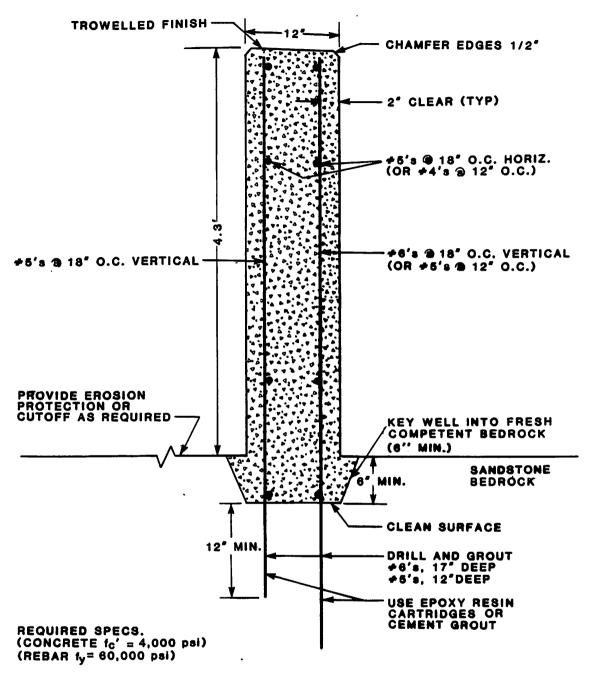
Plate 4 - Typical Section Concrete Wall Dam TPC-A

Appendix A - Hydrology and Hydraulic Calculations





VOLUME-ELEVATION CURVE TPC-A



(PROVIDE 1/2" EXPANSION JOINTS WITH 6" WATERSTOPS AT 10 FEET FROM EACH ABUTMENT)

SCALE: 1" = 1.0"

TYPICAL SECTION
CONCRETE WALL DAM
TPC-A

APPENDIX A HYDROLOGY AND HYDRAULIC CALCULATIONS

REVISIONS

BY _____ DATE ____ TO E0 ____

BY ____ DATE ____ TO E0 ____

TIME OF LODGENTRATION

ELEVATION DIFFERENCE = 25'

WATER COURSE LEDGLIH = 330' = .0625 mi

Tc = 0.031 hr

LAG TIME = 0.6Te = 0.018 hr

SCS CUEUG NUMBER

DRAINAGE COVER HYDROLOGIC SOIL WEIGHTED

AREA (ac) TYPE CONDITION TYPE CURVE NUMBER

2.5 disturbed — D 94

94

CHECKED BY 84/4 11/5/85

DRAINAGE BASIN AREA

2.5 ACRE 0,0039 SO MILE V

REVISIONS

UNIVERSAL SOIL LOSS EQUATION

RAINFALL FACTOR

· R= 40

SOIL ERODIBILITY FACTOR

SOIL TYPE =

SLOPE FACTOR

LENGTH(fi.) DELEV (fi.) SLOPE (%)

COVER FACTOR

ARGA (ac) OVER TYPE % COVER CANOPY (913) WEIGHTED

EROSION CONTROL FACTOR

P= 1.0

SEDIMENT INFLOW

A = 40 (.14) (1.68) 1.0) (1.0) = 9.41

ton /acre /year

 $A = 9.41 \left(\frac{1}{2047}\right)(2.5)(.95) = 0.011$

acre-feet /year

Dames & Moore