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Michael Schlenvogt

PEABODY WESTERN COAL COMPANY

For

Navajo County, Arizona

Kayenta Mine

N9-G

Temporary Sedimentation Structure

DESIGN REPORT

TABLE OF CONTENTS

Page

INTRODUCTION 1

INSPECTION 1

SITE DESCRIPTION 2

LAND USE 2

DESIGN ANALYSES 2

GENERAL 2

STABILITY 2

HYDROLOGY 3

HYDRAULICS 4

EMERGENCY SPILLWAY AND OUTLET CHANNEL 6

STORAGE CAPACITY 6

APPENDIX A -Hydrology, Hydraulic and Sedimentation Calculations

APPENDIX B -SEDCAD4 (Input and Output) 10-Year, 24-Hour Storm Event

APPENDIX C -SEDCAD4 (Input and Output) 25-Year, 6-Hour Storm Event

EXHIBIT #1 -N9-G Proposed Sedimentation Pond

The construction site of the proposed Structure N9-G was inspected in October, 2004 by a Registered Professional Engineer from Woodson Engineering, to assure that the site is suitable and no adverse conditions exist to prevent the successful construction of this structure. A detailed geotechnical investigation was not performed; rather, the information in Chapter 6, Attachment D was utilized for embankment design and will be utilized during construction to construct a stable embankment.

INSPECTION

This design report contains information specific to Structure N9-G. Mine-wide design, construction, and reclamation information is presented in the "General Report, Kayenta and Black Mesa Mines, Navajo County, Arizona, for Peabody Western Coal Company", December, 1985 (FAP), Chapter 6, Attachment D, Volume 2, along with the methods and results of analyses used for slope stability, hydrology, and hydraulics, and in Chapter 6, Pages 11 to 42, "Sediment and Water Control Facility Plan".

Sedimentation Structure N9-G will be an earthen embankment, designed and constructed by Peabody Western Coal Company (PWCC) as a temporary sedimentation structure to control runoff and sediment from portions of the N9 disturbed surface mining area at the Kayenta Mine. The location of Structure N9-G and its watershed boundary are shown on Drawing No. 85400, (Sheet K-6), and Drawing No. 85405. The site-specific general construction plans are shown on the attached Exhibit I.

INTRODUCTION

SITE DESCRIPTION

LAND USE

The N9-G Structure has a watershed of 102.94 acres and is located on a tributary upstream of Yazzie Wash at the Kayenta Mine. The 102.94-acre watershed, which contributes directly to structure N9-G, is classified as 21% undisturbed and 79% spoil.

DESIGN ANALYSES

GENERAL

Structure N9-G was designed under the supervision of a Registered Professional Engineer from Peabody Western Coal Company. The design was performed in accordance with applicable 30 CFR 780 and 816 regulations of the United States Department of Interior, Office of Surface Mining (OSM) and included a review of available project files. The most current information contained in the Peabody Western Coal Company files includes topographic maps developed from aerial photography flown in 1983 for Peabody Western Coal Company and was used in the analyses of the structure.

STABILITY

Structure N9-G is assumed to be a Category A-3 embankment. The structure will be a homogeneous earthen embankment, compacted in lifts to design specifications, and approximately 15 feet wide on top. A minimum upstream slope of 2H:1V and minimum downstream slope of 4H:1V were assumed. Based on the total embankment height of approximately 19.5 feet, these slopes are equal to or flatter than the recommended "worst case" embankment/foundation condition slopes in Table 3-6, Attachment D, Chapter 6; therefore, the embankment will be stable. The emergency spillway will be a minimum 20-foot wide, riprap-lined, trapezoidal channel.

HYDROLOGY

The hydrologic analysis was completed using the computer program SEDCAD4 (see Appendices A, B, and C). Structure N9-G is classified as a low hazard structure (see Drawing No. 85408). In addition, the mine area is sparsely populated with no one living in the downstream floodplain. The structure will impound less than 20 acre-feet and be less than 20 vertical feet in height from the upstream toe of the embankment of the natural stream elevation to the emergency spillway invert elevation. The structure has capacity less than 20 acre-feet; therefore, the spillway was analyzed using the 25-year, 6-hour storm event. The structure was conservatively assumed to be full to the emergency spillway at the time of 25-year storm event. The storage capacity of structure N9-G was analyzed using the 10-year, 24-hour storm event. The pond was verified to completely contain the 10-year, 24-hour storm event, and provide adequate sediment storage volume, without discharging into Yazzie Wash.

The following parameters were used in the hydrologic analysis:

1.	Water Course length, L	0.741 mi.
2.	Elevation Difference, H	190 ft
3.	Time of Concentration, T _c	0.244 hr
4.	NRCS Curve Number	85
5.	Rainfall Depth, 10-year, 24-hour storm 25-year, 6-hour storm	2.1 in 1.9 in
6.	Drainage Area	102.94 acres

Values reported represent the watershed, which drains directly to Pond N9-G.

HYDRAULICS

The SEDCAD4 and HYDROCALC computer programs were used to evaluate inflow to the sedimentation structure, outflow from the structure and the resulting water surface elevations. The initial conditions and results of the analysis are summarized in the following table (supporting calculations are presented in Appendices A, B, and C).

N9-G SEDIMENTATION POND HYDRAULICS TABLE

Units	10-Yr. 24-Hr Storm	25-Yr. 6-Hr Storm
Initial Reservoir Volume Condition		
	Empty	Full to emergency spillway
Inflow		
Peak Flow	76.14	89.95
Volume	7.46	6.2
Storage		
Peak Stage	N/A	6677.0
Emerg. Spillway Elev.	6676.1	6676.1
Peak Storage	N/A	21.61
Storage Capacity	19.38	19.38
Outflow		
Peak Flow	N/A	33.19
Spillway Elevation	6676.1	6676.1
Embankment Crest Elev.	6679.9	6679.9
Peak Stage	--	6677.0
Freeboard	--	2.9
Emergency Spillway Channel		
Flow Depth	--	0.9
Critical Velocity	--	4.88
Mannings "n"	--	0.048
Width	--	20
Outflow Channel		
Slope	--	25
Normal Velocity	--	7.61
Normal Depth	--	0.6
Mannings "n"	--	0.062
Riprap D ₅₀	--	6

1	Rainfall Factor, R	40
2	Soil Erodibility Factor, K	0.13
3	Slope Factor, LS	8.89
4	Cover Factor, C	0.83
5	Erosion Control Factor, P	0.77

Universal Soil Loss Equation with the following parameters:

The calculations for the sediment load entering structure N9-G were made utilizing the Revised

approximately 19.38 acre-foot.

The impoundment stage-capacity table (see Exhibit 1) is based on the 1983 aerial topographic mapping conducted for Peabody Western Coal Company. Structure N9-G is designed to contain

STORAGE CAPACITY

as a transition into the downstream channel.

A minimum 15-foot long riprap-lined channel will be constructed beyond the toe of the embankment

Minimum Channel Depth	(Spillway)	2.0	ft
Channel Width		20	ft
Channel Length	(Spillway)	38	ft
	(Outflow)	81	ft
Sideslopes (Horizontal to Vertical)		3:1	or flatter
Average Slope	(Spillway)	0	%
Maximum Slope	(Outflow)	25	%
Spillway Elevation		6676.1	ft

The emergency spillway and outlet channel for N9-G will be a trapezoidal channel with dimensions listed below. The alignment and dimensions are shown on Exhibit 1.

EMERGENCY SPILLWAY AND OUTLET CHANNEL

Exhibit #1 - N9-G Proposed Sedimentation Pond

Appendix C - SEDCAD4 (Input and Output) 25-Year, 6-Hour Storm Event

Appendix B - SEDCAD4 (Input and Output) 10-Year, 24-Hour Storm Event

Appendix A - Hydrology, Hydraulic, and Sedimentation Calculations

The following appendices and drawing are attached and complete this design report.

* * *

N9-G	
Total Storage Capacity (ac-ft)	19.38
10-Year, 24-Hour Storm Inflow (ac-ft)	7.46
Available Sediment Storage Capacity (ac-ft)	11.92
Sediment Inflow Rate/Year (ac-ft/yr)	1.32
Sediment Storage Life (yr)	9.0

Storage for Structure N9-G

The hydrologic analysis gives the storage volume required to contain the 10-year, 24-hour storm, and the remaining storage volume available for storing sediment. Structure N9-G has sufficient storage capacity to contain the 10-year, 24-hour storm event. The storage capacity was determined for N9-G Structure and the results of the analysis are presented in the following table.

Hydrology, Hydraulic, and Sedimentation Calculations

APPENDIX A

**PEABODY WESTERN COAL COMPANY
CALCULATED HYDROLOGIC DATA**

PROJECT: N-9 MINING AREA

STRUCTURE: G

TIME OF CONCENTRATION:

Start Elevation (ft) = 6855
 End Elevation (ft) = 6665
 Elevation Difference, E (ft) = 190
 Watercourse Length (ft) = 3915
 Watercourse Length (mi) = 0.741
 $T_c = (11.9L^{0.73}/E)^{0.385} = 0.244$ hours

ROUTING PARAMETERS:

Between structure routing parameters were calculated using the SCS Upland method in SEDCAD4. Input and output parameters are shown on the SEDCAD4 printouts in Appendices C.

SCS CURVE NUMBER:

Cover Type	Soil Group	Curve Number	Area (acres)	CN*Area
Pinyon Juniper	B	65	3.803	247.195
Pinyon Juniper	D	83	3.46	287.18
Sagebrush	B	60	2.284	137.04
Disturbed Land	B	86	91.194	7842.684
Disturbed Land	D	86	2.194	188.684
TOTAL:				8702.783

Weighted CN = Total CN * Area / Total Area =

85

Note: During Operations the open pit will collect most of the runoff from disturbed areas. The worst case is when the open pit is reclaimed and runoff from a larger area or regraded soil reports directly to the pond.

DRAINAGE BASIN AREA:

102.935 Acres

PEARBODY WESTERN COAL COMPANY
CALCULATED SEDIMENTOLOGY DATA

STRUCTURE: G

SOIL ERODIBILITY FACTOR:

Soil Type	Erodibility Factor, K	Area	K*Area
13A	0.37	4.528	1.68
14AB	0.37	2.346	0.87
16F	0.05	8.232	0.41
3F	0.02	3.109	0.06
16CE	0.05	1.065	0.05
1AB	0.16	0.851	0.14
3CD	0.16	1.694	0.27
Disturbed	0.12	81.11	9.73
TOTAL			13.21
		102.935	

Weighted K = Total K * Area / Total Area =

0.13

SLOPE FACTOR:

Length (ft)	Slope (%)	m	Slope Angle (deg)	LS Factor
352	22.10%	0.60	11.56	7.39
417	7.80%	0.50	13.87	6.28
648	44.40%	0.60	15.28	14.60
1018	30.10%	0.60	6.83	7.30

Average LS = 8.89

The LS Factor was calculated by:

LS=(Slope Length/72.6)^m*(10.8sin(slope angle)+.03) for slopes < 9%
 LS=(Slope Length/72.6)^m*(16.8sin(slope angle)-0.5) for slopes > 9%

Where:

- Slope < 3% m = 0.3
- Slope = 4% m = 0.4
- 5% < Slope < 10% m = 0.5
- Slope > 10% m = 0.6

STRUCTURE: G

Cover and Practice Factors:

Cover Type	Cover (%)	Canopy (%)	Area (acres)	Cover Factor, C	C * Area	Practice Factor, P	P * Area
Pinyon Juniper	40%	25%	7.263	0.22	1.60	1.00	7.263
Sagebrush, Grass	40%	25%	2.284	0.2	0.46	1.00	2.284
Saltbrush	40%	25%	0	0.2	0.00	1.00	0
Reclaimed	40%	25%	12.278	0.15	1.84	0.40	4.9112
Disturbed	0%	0%	81.11	1	81.11	0.80	64.888
TOTAL:			102.935		85.01		79.35

Weighted C = Total C * Area / Total Area = 0.83

Weighted P = Total P * Area / Total Area = 0.77

RAINFALL FACTOR:

R = 40

**PEABODY WESTERN COAL COMPANY
CALCULATED SEDIMENT YIELD**

STRUCTURE: G

The following spreadsheet calculates the predicted sediment yield for the project area. The gross sediment yield is determined according to the Revised Universal Soil Loss Equation.

PARAMETER DESCRIPTION	VALUE
Annual Rainfall Factor (R)	40.00
Soil Erodibility Factor (K)	0.13
Length Slope Factor (L)	8.89
Cover Factor(C)	0.83
Practice Factor (P)	0.77
Gross Annual Sediment Yield	29.07 tons/acre/year
Sediment Density	94.00 pcf
Gross Annual Sediment Yield	0.0142 acre-feet/acre/year
Sediment Delivery Ratio	90%
Estimated Annual Sediment Yield	0.0128 acre-feet/acre/year
Watershed Area	102.94 acres
Watershed Annual Sediment Yield	1.32 acre-feet/year
Number of Years	1.00 years
Calculated Sediment Volume	1.32 acre-feet

TRAPEZOIDAL CHANNEL ANALYSIS
 CRITICAL DEPTH COMPUTATION
 N9-G POND
 November 8, 2004

PROGRAM INPUT DATA

DESCRIPTION	VALUE
Flow Rate (cfs).....	89.95
Channel Bottom Slope (ft/ft).....	0.005
Manning's Roughness Coefficient (n-value).....	0.048
Channel Left Side Slope (horizontal/vertical).....	3.0
Channel Right Side Slope (horizontal/vertical).....	3.0
Channel Bottom Width (ft).....	20.0

COMPUTATION RESULTS

DESCRIPTION	VALUE
Critical Depth (ft).....	0.82
Critical Slope (ft/ft).....	0.0377
Flow Velocity (fps).....	4.88
Froude Number.....	1.0
Velocity Head (ft).....	0.37
Energy Head (ft).....	1.19
Cross-Sectional Area of Flow (sq ft).....	18.44
Top Width of Flow (ft).....	24.92

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SEDCAD4 (Input and Output) 10-Year, 24-Hour Storm Event

APPENDIX B

Peabody Western Coal
Kayenta Mine
N9-G POND DESIGN
10YR 24HR STORM

Gary Altsisi

General Information

Storm Information:

Storm Type:	NRCS Type II
Design Storm:	10 yr - 24 hr
Rainfall Depth:	2.100 Inches

Structure Summary:

# 1	102,940	102,940	76.14	7.46
Immediate	Contributing	Area	Area	Peak
Contributing	Area	Area	Discharge	Runoff
(ac)	(ac)	(ac)	(cfs)	(ac-ft)
Total	Contributing	Area	Peak	Total
102,940	102,940	102,940	76.14	7.46

Subwatershed Hydrology Detail:

Stru #	SWS #	SWS Area (ac)	Time of Conc (hrs)	Musk K (hrs)	Musk X	Curve Number	UHS	Peak Discharge (cfs)	Runoff Volume (ac-ft)
#1	1	102.940	0.244	0.000	0.000	85.000	F	76.14	7.456
		Σ						76.14	7.456

SEDCAD4 (Input and Output) 25-Year, 6-Hour Storm Event

APPENDIX C

Peabody Western Coal
Kayenta Mine
N9-G POND DESIGN
25YR 6HR STORM

Gary Altsisi

General Information

Storm Information:

Storm Type:	NRCS Type II
Design Storm:	25 yr - 6 hr
Rainfall Depth:	1.900 inches

Subwatershed Hydrology Detail:

Stru #	SWS #	SWS Area (ac)	Time of Conc (hrs)	Musk K (hrs)	Musk X	Curve Number	UHS	Peak Discharge (cfs)	Runoff Volume (ac-ft)
#1	1	102.940	0.244	0.000	0.000	85.000	F	89.95	6.200
Σ		102.940						89.95	6.200

Structure Networking:

Type	Stru #	(flows into)	Stru #	Musk. K	Musk. X	Description
Pond	#1	>=	End	0.000	0.000	N9-G

#1
Pond

Structure Summary:

	In	Out	# 1
Immediate Contributing Area (ac)	102.940	102.940	
Total Contributing Area (ac)	89.95	33.19	
Peak Discharge (cfs)	6.20	6.20	
Total Runoff Volume (ac-ft)			

Structure Detail:

Structure #1 (Pond)

N9-G

Pond Inputs:

Initial Pool Elev:	6,676.10
Initial Pool:	19.38 ac-ft

Emergency Spillway

Spillway Elev	6,676.10	Crest Length (ft)	38.40	Left Slope	3.00:1	Right Slope	3.00:1	Bottom Width (ft)	20.00
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Pond Results:

Peak Elevation:	6,676.96
Dewater Time:	0.36 days

Dewatering time is calculated from peak stage to lowest spillway

Elevation-Capacity-Discharge Table

Elevation	Area (ac)	Capacity (ac-ft)	Discharge (cfs)	Dewater Time (hrs)
6,660.00	0.099	0.000	0.000	0.000
6,660.50	0.139	0.059	0.000	0.000
6,661.00	0.186	0.140	0.000	0.000
6,661.50	0.239	0.247	0.000	0.000
6,662.00	0.299	0.381	0.000	0.000
6,662.50	0.366	0.547	0.000	0.000
6,663.00	0.439	0.748	0.000	0.000
6,663.50	0.520	0.987	0.000	0.000
6,664.00	0.606	1.269	0.000	0.000
6,664.50	0.700	1.595	0.000	0.000
6,665.00	0.800	1.970	0.000	0.000
6,665.50	0.840	2.380	0.000	0.000
6,666.00	0.881	2.810	0.000	0.000
6,666.50	0.922	3.260	0.000	0.000
6,667.00	0.965	3.732	0.000	0.000
6,667.50	1.009	4.226	0.000	0.000
6,668.00	1.053	4.741	0.000	0.000

Elevation	Area (ac)	Capacity (ac-ft)	Discharge (cfs)	Dewater Time (hrs)
6,668.50	1.099	5.279	0.000	
6,669.00	1.146	5.840	0.000	
6,669.50	1.193	6.425	0.000	
6,670.00	1.242	7.034	0.000	
6,670.50	1.360	7.684	0.000	
6,671.00	1.483	8.394	0.000	
6,671.50	1.611	9.167	0.000	
6,672.00	1.745	10.006	0.000	
6,672.50	1.884	10.913	0.000	
6,673.00	2.029	11.891	0.000	
6,673.50	2.178	12.943	0.000	
6,674.00	2.333	14.070	0.000	
6,674.50	2.494	15.277	0.000	
6,675.00	2.659	16.565	0.000	
6,675.50	2.836	17.937	0.000	
6,676.00	3.025	19.394	0.000	
6,676.10	2.457	19.379	0.000	Spillway #1
6,676.50	2.597	20.389	15.508	7.20
6,676.96	2.761	21.614	33.190	1.45 Peak Stage
6,677.00	2.777	21.732	34.898	
6,677.50	2.963	23.166	83.680	
6,678.00	3.155	24.695	146.943	
6,678.50	3.353	26.322	222.558	
6,679.00	3.557	28.049	319.433	
6,679.50	3.766	29.879	435.004	
6,680.00	3.983	31.817	573.728	

Detailed Discharge Table

Combined	Total	Emergency Spillway (cfs)	Discharge (cfs)
6,660.00	0.000	0.000	0.000
6,660.50	0.000	0.000	0.000
6,661.00	0.000	0.000	0.000
6,661.50	0.000	0.000	0.000
6,662.00	0.000	0.000	0.000
6,662.50	0.000	0.000	0.000
6,663.00	0.000	0.000	0.000
6,663.50	0.000	0.000	0.000

Combined	Total	Elevation	Emergency	Spillway (cfs)	Discharge	(cfs)
6,664.00	0.000	6,664.00	0.000	0.000	0.000	0.000
6,664.50	0.000	6,664.50	0.000	0.000	0.000	0.000
6,665.00	0.000	6,665.00	0.000	0.000	0.000	0.000
6,665.50	0.000	6,665.50	0.000	0.000	0.000	0.000
6,666.00	0.000	6,666.00	0.000	0.000	0.000	0.000
6,666.50	0.000	6,666.50	0.000	0.000	0.000	0.000
6,667.00	0.000	6,667.00	0.000	0.000	0.000	0.000
6,667.50	0.000	6,667.50	0.000	0.000	0.000	0.000
6,668.00	0.000	6,668.00	0.000	0.000	0.000	0.000
6,668.50	0.000	6,668.50	0.000	0.000	0.000	0.000
6,669.00	0.000	6,669.00	0.000	0.000	0.000	0.000
6,669.50	0.000	6,669.50	0.000	0.000	0.000	0.000
6,670.00	0.000	6,670.00	0.000	0.000	0.000	0.000
6,670.50	0.000	6,670.50	0.000	0.000	0.000	0.000
6,671.00	0.000	6,671.00	0.000	0.000	0.000	0.000
6,671.50	0.000	6,671.50	0.000	0.000	0.000	0.000
6,672.00	0.000	6,672.00	0.000	0.000	0.000	0.000
6,672.50	0.000	6,672.50	0.000	0.000	0.000	0.000
6,673.00	0.000	6,673.00	0.000	0.000	0.000	0.000
6,673.50	0.000	6,673.50	0.000	0.000	0.000	0.000
6,674.00	0.000	6,674.00	0.000	0.000	0.000	0.000
6,674.50	0.000	6,674.50	0.000	0.000	0.000	0.000
6,675.00	0.000	6,675.00	0.000	0.000	0.000	0.000
6,675.50	0.000	6,675.50	0.000	0.000	0.000	0.000
6,676.00	0.000	6,676.00	0.000	0.000	0.000	0.000
6,676.10	0.000	6,676.10	0.000	0.000	0.000	0.000
6,676.50	15.508	6,676.50	15.508	0.000	15.508	15.508
6,677.00	34.898	6,677.00	34.898	0.000	34.898	34.898
6,677.50	83.680	6,677.50	83.680	0.000	83.680	83.680
6,678.00	146.943	6,678.00	146.943	0.000	146.943	146.943
6,678.50	222.558	6,678.50	222.558	0.000	222.558	222.558
6,679.00	319.433	6,679.00	319.433	0.000	319.433	319.433
6,679.50	435.004	6,679.50	435.004	0.000	435.004	435.004
6,680.00	573.728	6,680.00	573.728	0.000	573.728	573.728