INSPECTION REPORT

Sedimentation Structure

N5-E

Black Mesa Mine

Navajo County, Arizona

for

PEABODY COAL COMPANY

Dames & Moore
10139-011-22

TABLE OF CONTENTS

INTRODUCTION

Sedimentation Structure N5-E is a partially incised structure with an earthen embankment, designed and constructed in 1983 by Peabody Coal Company as a temporary sedimentation structure to control runoff and sediment from the disturbed mining areas of the Black Mesa Mine. The location of Structure N5-E is shown on Plate 1, Site Plan.

This inspection report contains information specific to Structure N5-E. Regional site information is presented in the "General Report, Kayenta and Black Mesa Mines, Navajo County, Arizona for Peabody Coal Company," along with the methods and results of analyses used for slope stability, hydrology and hydraulics.

INSPECTION

Structure N5-E was inspected on September 4, 1985 by an interdisciplinary team of engineers from Dames & Moore. The purpose of the inspection was to assess the safety and general condition of the structure with respect to United States Department of Interior, Office of Surface Mining (OSM) regulations.

Dames & Moore's inspection was performed in accordance with applicable 30 CFR 780 and 816 regulations and included a review of the N5-E project files and a field inspection of the structure. The most current information contained in the Peabody Coal Company files includes the 1984 and current survey data and inspections performed in 1984 and 1985 by

Peabody Coal Company. The survey data developed in August 1984 was used in the analyses of the structure. Results of the field inspection are included in this report as Appendix A.

SITE DESCRIPTION

LAND USE

Structure N5-E has a 15.1-acre tributary drainage area and is located near Coal Mine Wash at the Black Mesa Mine. The watershed is classified as 100% disturbed.

EMBANKMENT

Structure N5-E is a homogeneous earthen embankment classified as a sidehill embankment. Physical characteristics of the embankment are listed in the following table:

Structure N5-E

Embankment	Residual Sandstone Soils
Foundation	Alluvium
Right Abutment	Residual Sandstone Soils
Left Abutment	Residual Sandstone Soils
Height	7.2 ft
Crest Width	18 ft
Upstream Slope	3.1 H : 1 V
Downstream Slope	2.1 H : 1 V

A cross-section of the embankment is shown on Plate 2, Existing Maximum Cross Section N5-E, A-A'.

ANALYSES

STABILITY

Structure N5-E is a category A-3 embankment. A standard category A-3 embankment has static and seismic factors of safety equal to or greater than 1.5 and 1.2, respectively, under the following conditions:

1. Maximum height = 10 ft
2. Maximum upstream slope = 1.5 H : 1 V
3. Maximum downstream slope = 2.5 H : 1 V
4. Normal pool with steady seepage saturation conditions

The N5-E embankment is lower in height; however, the downstream slope is steeper than the category standard; therefore, the embankment has factors of safety less than the design minimum.

HYDROLOGY

The hydrologic analysis was completed using the U.S. Army Corps of Engineers generalized computer program HEC-1, Flood Hydrograph Package. Structure N5-E is located downstream from Structure N5-D. The two structures have a combined storage capacity that is less than 20 acre-feet. Therefore, the spillway for N5-D was analyzed using the 25-year, 6-hour storm. The storage capacity of Structure N5-E was analyzed using the 10-year, 24-hour storm.

The following parameters were used in the hydrologic analysis:

1.	Water Course length, L	0.152	mi
2.	Assumed Water Course Slope	4	%
3.	Average Channel Velocity	3	fps
4.	Time of Concentration, T_c	0.074	h
5.	Lag time, $0.6T_c$	0.044	h
6.	SCS Curve Number	94	
7.	Rainfall Depth, 10-year, 24-hour storm	2.1	in.
	25-year, 6-hour storm	1.9	in.
8.	Drainage Area	15.1	acres

Drainage basin is now an active mine area. Topography is not available to calculate time of concentration using standard methods. An assumed water course length, slope and velocity were used to estimate T_c.

HYDRAULICS

The HEC-1 program was used to evaluate inflow to the sedimentation structure, outflow from the structure and the resulting water surface elevations. Both the 10-year and 25-hear storms were routed through Structure N5-D and into Structure N5-E. The initial conditions and results of the analysis are summarized in the following table.

	Units	10-year 24-hour Storm	25-year 6-hour Storm
Initial Reservoir Volume Condition		Empty	Full to the spillway elevation
Inflow			
Peak Flow	cfs	46	63
Volume	acre-ft	1.89	1.61
Storage			
Peak Stage	ft	6463.82	6464.22
Spillway Elevation . .	ft	6462.53	--
Peak Storage	acre-ft	--	--
Storage Capacity . . .	acre-ft	3.43	--
Outflow			
Peak Flow	cfs	10	47
Embankment Crest Elevation	ft	--	6464.53
Peak Stage	ft	--	6464.22
Freeboard	ft	--	0.31

Spillway Channel

The existing spillway for N5-E has a trapezoidal channel with the following dimensions:

Channel depth	2 ft
Channel width	18 ft
Channel length	42 ft
Side slopes (horizontal to vertical)	2:1
Average exit slope	0 percent

There is presently no erosion protection within the channel.

Outflow Channel

The structure presently has no outflow channel.

STORAGE CAPACITY

The impoundment volume-elevation curve is based on site specific surveys conducted for Peabody Coal Company's August 1984 inspection, and 1985 resurveys, where available. Additionally, the most current topographic maps available were used in developing Plate 3, Volume-Elevation Curve, N5-E.

The calculations for the sediment load entering Structure N5-E were made utilizing the Universal Soil Loss Equation with the following parameters:

1. Rainfall Factor, R 40
2. Soil Erodibility Factor, K 0.16
3. Slope Factor, LS 2.72
4. Cover Factor, C 1.00
5. Erosion Control Factor, P 1.0

The hydrologic analysis gives the storage volume required to contain the 10-year, 24-hour storm, and the remaining storage volume available for storing sediment. The existing storage capacity of N5-E and the results of the sediment inflow analysis are summarized in the following table.

N5-E STORAGE

Total Storage Capacity	3.43	acre-ft
10-year, 24-hour Storm Inflow	1.89	acre-ft
Available Sediment Storage Capacity . .	0	acre-ft
Sediment Inflow Rate	0.122	acre-ft/yr
Sediment Storage Life	0	yrs

REMEDIAL COMPLIANCE PLAN

GEOTECHNICS

The inspection of Structure N5-E indicated that the only geotechnical problem is rill erosion on the upstream slope and the side slopes of the spillway channel. Correction of erosion is considered a periodic maintenance task and does not require remedial action. The downstream slope should be flattened to 2.5 horizontal to 1 vertical to meet stability requirements.

HYDRAULICS

The storage capacity and spillway capacity of Structure N5-E are inadequate. The structure does not have an outflow channel. The storage capacity should be increased to 8.7 acre-feet by excavating the pond as shown on Plates 1 and 4. The embankment crest should be raised to elevation 6465.30 feet. A trapezoidal outflow channel should be constructed along the alignment shown in Plate 1. The channel profile is shown in Plate 4 and the required dimensions are shown in Plate 5. Both the spillway and outflow channel should be protected against erosion using geotextile and riprap as shown in Plate 5.

The hydraulic analysis of the excavated impoundment is summarized in the following table.

N5-E HYDRAULICS FOR EXCAVATED IMPOUNDMENT

	Units	10-year 24-hour Storm	25-year 6-hour Storm	
Initial Reservoir Volume Condition		Empty	Full to the spillway elevation	
Inflow				
Peak Flow	cfs	46	136	
Volume	acre-ft	1.89	1.61	
Storage				
Peak Stage	ft	6458.07	--	
Spillway Elevation	ft	6462.53	--	
Peak Storage	acre-ft	4.84	--	
Storage Capacity	acre-ft	8.7	--	
Outflow				
Peak Flow	cfs	0	133	
Embankment Crest Elevation	ft	--	6465.30	
Peak Stage	ft	--	6464.30	
Freeboard	ft	--	1.00	
Spillway Channel				
Flow Depth	ft	--	1.77	
Critical Velocity	fps	--	5.0	
Manning's "n"		--	0.040	
Outflow Channel			Section I	Section II
Slope	%	--	8	21
Normal Velocity	fps	--	7.1	9.6
Normal Depth	ft	--	0.59	0.44
Manning's "n"		--	0.040	0.040

The hydraulic analysis gives the storage volume required to contain the 10-year, 24-hour storm, and the remaining storage volume available for storing sediment. Structure N5-E is located downstream from Structure N5-D and receives excess flow from N5-D during the 10-year, 24-hour storm. Therefore, the storage capacity of the two structures must be combined to determine the available sediment storage life. The 10-year, 24-hour storm was routed through N5-D and N5-E. The results of this analysis are summarized below.

N5-D AND N5-E COMBINED STORAGE

	N5-D	N5-E	Total	
Total Storage Capacity	5.36	8.70	14.06	acre-ft
10-year, 24-hour Storm Inflow	8.48	1.89	10.37	acre-ft
Available Sediment Storage Capacity . .	--	--	3.69	acre-ft
Sediment Inflow Rate	0.27	0.122	0.392	acre-ft/yr
Sediment Storage Life	--	—	9	yrs

* * *

The following plates and appendix are attached and complete this inspection report.

Plate 1 - Site Plan N5-E

Plate 2 - Existing Maximum Cross Section N5-E, A-A'

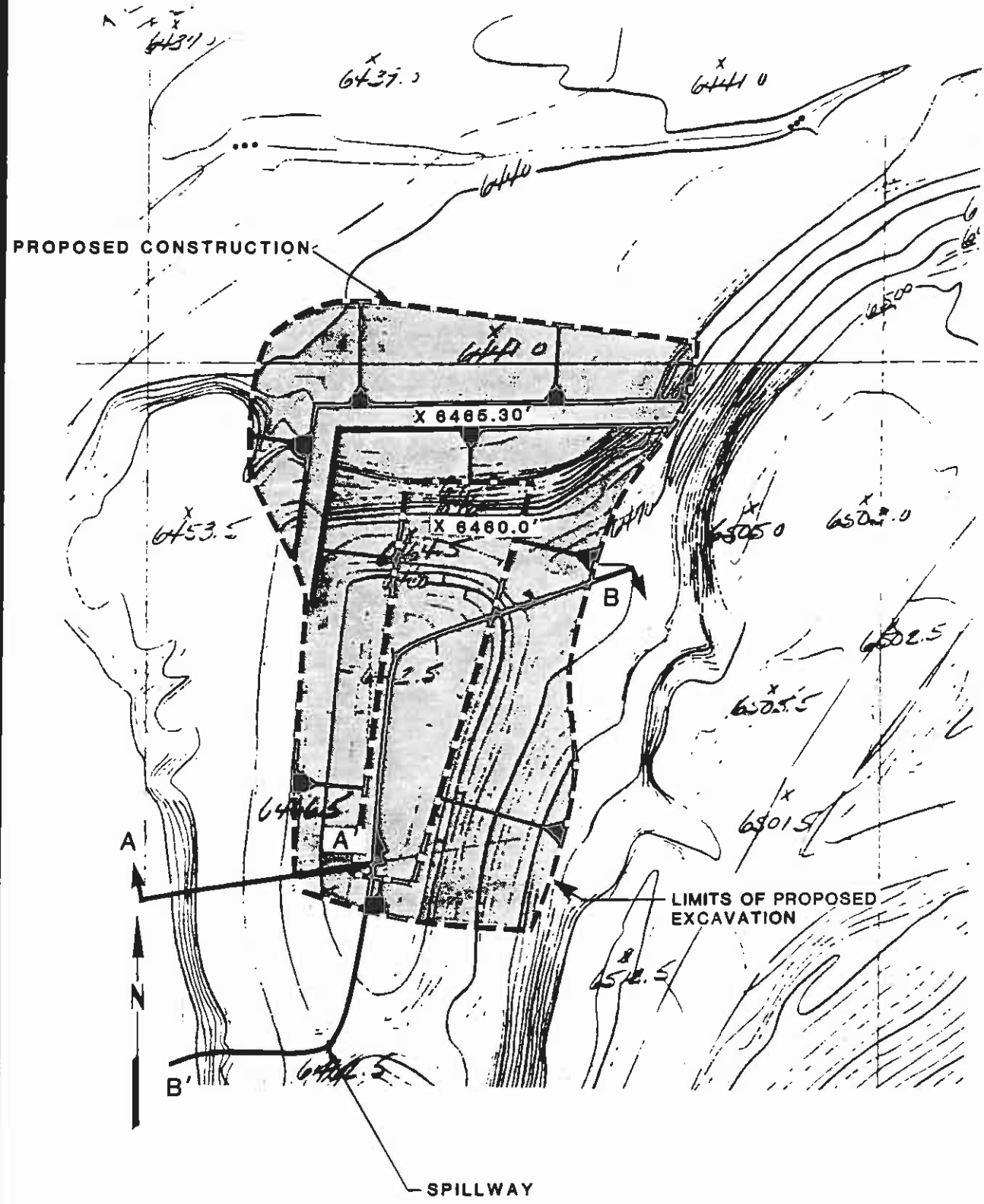
Plate 3 - Volume-Elevation Curve N5-E

Plate 4 - Channel Profile N5-E, B-B'

Plate 5 - Spillway and Outflow Channel Cross Section N5-E

Appendix A - Inspection Check List

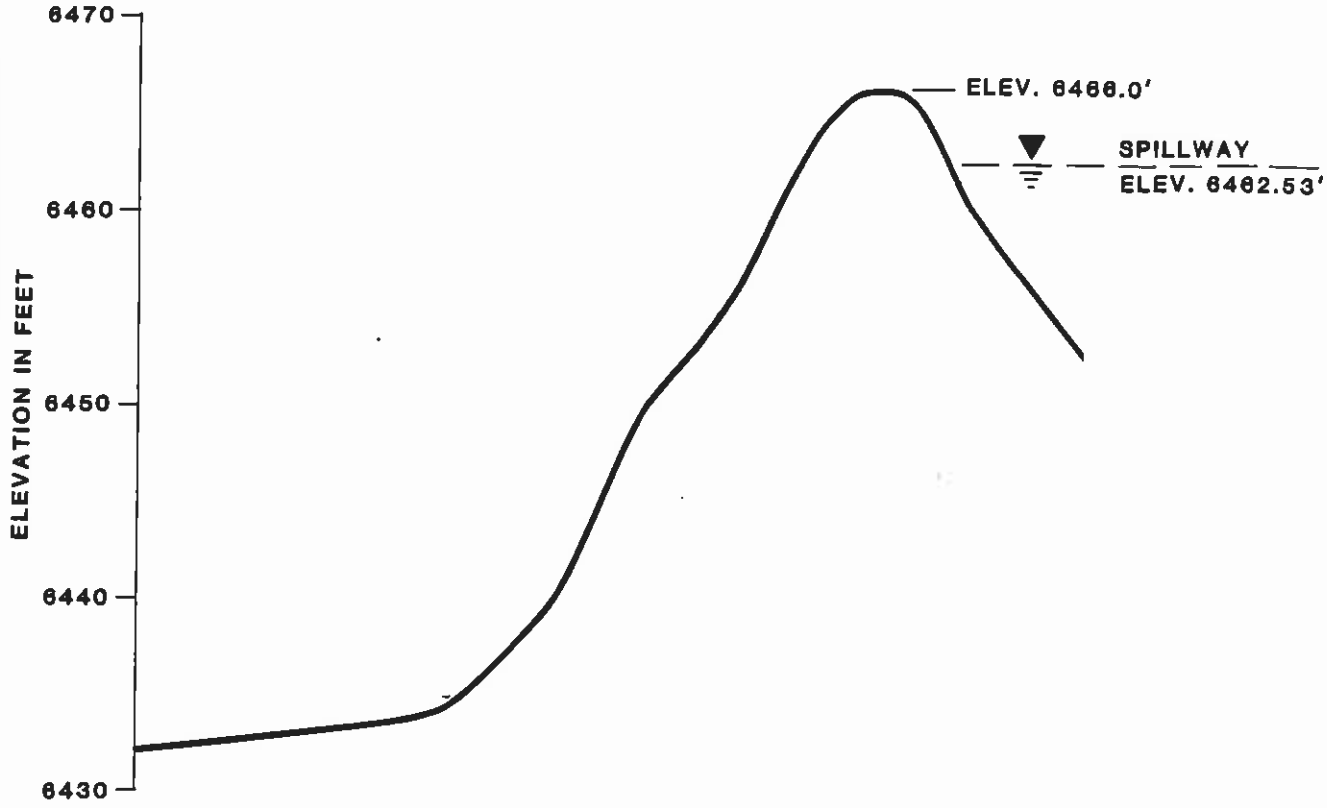
Appendix B - Hydrology and Hydraulic Calculations



SCALE
0 100 200
FEET

SITE PLAN
N5-E

BY **Dames & Moore** Plate 1



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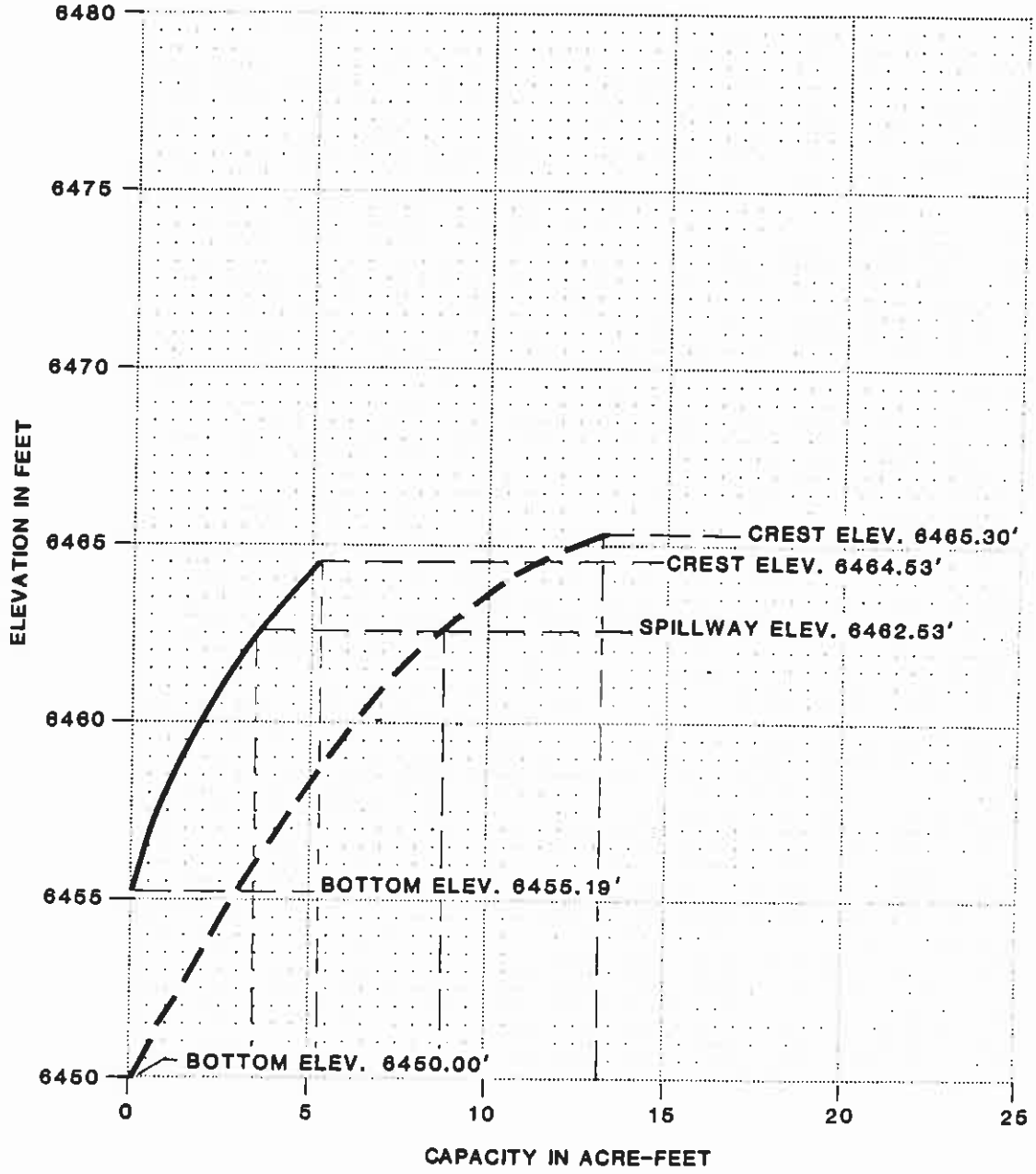
FEET

EXISTING MAXIMUM CROSS-SECTION A-A' N5-E

FOR LOCATION SEE PLATE 1

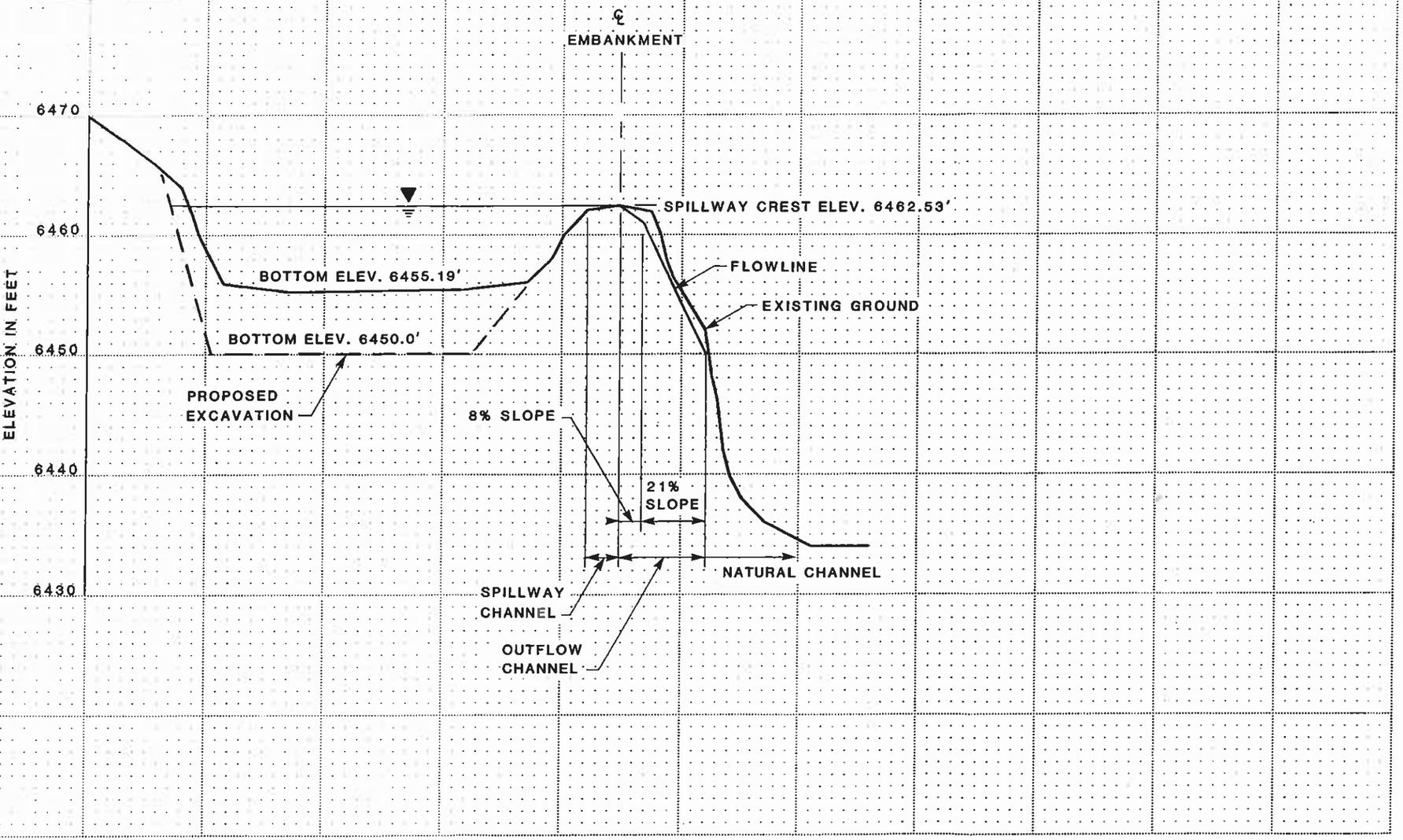
BY **Dames & Moore** Plate 2



EXISTING VOLUME
PROPOSED VOLUME

VOLUME-ELEVATION CURVE N5-E

BY **Dames & Moore** Plate 3

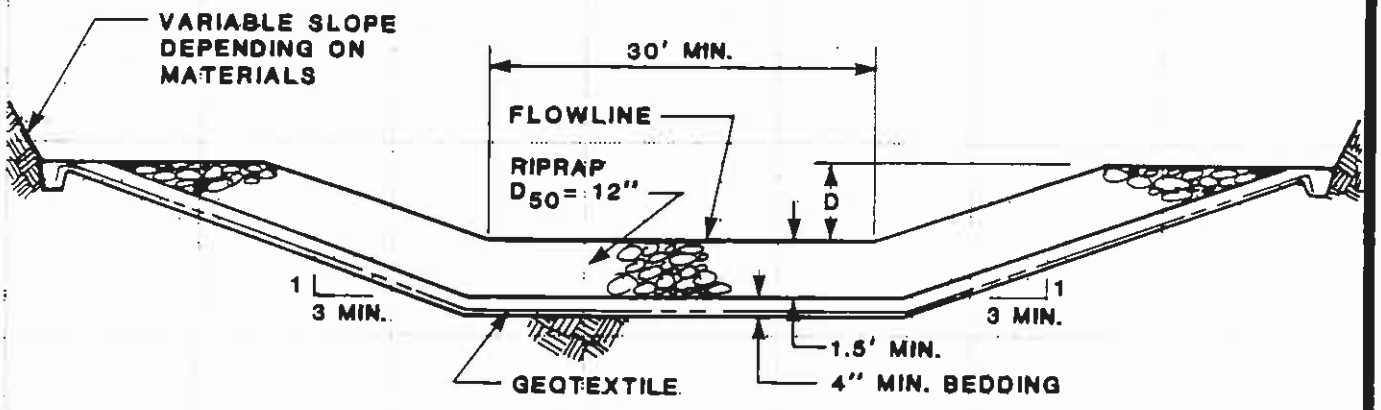


SCALE
0 100 200
FEET

CHANNEL PROFILE B-B'
N5-E

FOR LOCATION SEE PLATE 1

BY **Dames & Moore** Plate 4



SPILLWAY CHANNEL

D = 2.8'

LENGTH = 30'

FLOWLINE ELEV. = 6462.53'

OUTFLOW CHANNEL

D = 1.5'

SPILLWAY AND OUTFLOW CHANNEL CROSS SECTION N5-E

BY **Dames & Moore** Plate 5

APPENDIX A

INSPECTION CHECK LIST

INSPECTION CHECK LIST

ITEM	YES	NO	REMARKS
1. CREST			
a. Any visual settlements?		X	
b. Misalignment?		X	
c. Cracking?		X	
2. UPSTREAM SLOPE			
a. Adequate grass cover?		X	
b. Any erosion?	X		Minor Rills
c. Are trees growing on slope?		X	
d. Longitudinal cracks?		X	
e. Transverse cracks?		X	
f. Adequate riprap protection?	X		
g. Any stone deterioration?			NA
h. Visual depressions or bulges?		X	
i. Visual settlements?		X	
j. Animal burrows?		X	
3. DOWNSTREAM SLOPE			
a. Adequate grass cover?		X	
b. Any erosion?	X		
c. Are trees growing on slope?		X	
d. Longitudinal cracks?		X	
e. Transverse cracks?		X	
f. Visual depressions or bulges?		X	
g. Visual settlements?		X	
h. Is the toe drain dry?			NA
i. Are the relief wells flowing?			NA
j. Are boils present at the toe?		X	
k. Is seepage present?		X	
l. Animal burrows?		X	
4. ABUTMENT CONTACT. RIGHT			
a. Any erosion?		X	
b. Visual differential movement?		X	
c. Any cracks noted?		X	
d. Is seepage present?		X	
e. Type of Material?			reddish brown SM
5. ABUTMENT CONTACT. LEFT			
a. Any erosion?		X	
b. Visual differential movement?		X	
c. Any cracks noted?		X	
d. Is seepage present?		X	
e. Type of Material?			reddish brown SM

ITEM	YES	NO	REMARKS
6. SPILLWAY/NORMAL			
a. Location:			
Left abutment?			
Right abutment?			
Crest of Embankments?	X		Adjacent to LA
b. Approach Channel:		X	
Are side slopes eroding?			NA
Are side slopes sloughing?			
Bottom of channel eroding?			
Obstructed?			
Erosion protection?			
c. Spillway Channel:	X		
Are side slopes eroding?	X		Minor Rills on Abut 3. Lt
Are side slopes sloughing?		X	
Bottom of channel eroding?		X	
Obstructed?		X	
Erosion protection?		X	
d. Outflow Channel:		X	
Are side slopes eroding?			NA
Are side slopes sloughing?			
Bottom of channel eroding?			
Obstructed?			
Erosion protection?			
e. Weir:		X	
Condition?			
7. SPILLWAY/EMERGENCY			NA
a. Location:			
Left abutment?			
Right abutment?			
Crest of Embankments?			
b. Approach Channel:			
Are side slopes eroding?			
Are side slopes sloughing?			
Bottom of channel eroding?			
Obstructed?			
Erosion protection?			
c. Spillway Channel:			
Are side slopes eroding?			
Are side slopes sloughing?			
Bottom of channel eroding?			
Obstructed?			
Erosion protection?			
d. Outflow Channel:			
Are side slopes eroding?			
Are side slopes sloughing?			
Bottom of channel eroding?			
Obstructed?			
Erosion protection?			
e. Weir:			
Condition?			

ITEM	YES	NO	REMARKS
8. IMPOUNDMENT			
a. Sinkholes?		X	(Elev.) feet
b. Water present?	X		(Elev.) feet
c. Siltation?	X		
d. Watershed matches soil map?		X	

9. GENERAL COMMENTS

APPENDIX B

HYDROLOGY AND HYDRAULIC CALCULATIONS

FILE PEABODY COAL CO. 10139-011-22
SUBJECT SEDIMENT POND HYDROLOGY
N5-E SHEET ___ OF ___

TIME OF CONCENTRATION

~~ELEVATION DIFFERENCE =~~

~~WATER COURSE LENGTH~~

SPECIAL CASE: BASIN IS NOW A PIT THAT WILL EVENTUALLY BE RECLAIMED. ASSUME MAX. LENGTH = 300' AND MAXIMUM SLOPE = 4%. USE VELOCITY = 3 ft/s

$T_c = \frac{300}{3} = 267 \text{ sec} = 0.074 \text{ hr}$

LAG TIME = $0.6 T_c = 0.044$ hr.

REVISIONS
BY ___ DATE ___ TO EO ___
BY ___ DATE ___ TO EO ___

SCS CURVE NUMBER

DRAINAGE AREA (ac)	COVER TYPE	HYDROLOGIC CONDITION	SOIL TYPE	WEIGHTED CURVE NUMBER
100%	DISTURBED	—	D	94

E.T #24 before disturbance — USE 91

DRAINAGE BASIN AREA

15.1 ACRE 0.024 SQ MILE

BY ___ DATE ___
CHECKED BY ___
COPY TO EO ___

Dames & Moore

945 1 (3-72) PRINTED IN U S A

FILE PEABODY COAL CO 10139-011-22
SUBJECT SEDIMENT INFLOW
N5-E SHEET ___ OF ___

REVISIONS BY ___ DATE ___ TO EO ___ BY ___ DATE ___ TO EO ___

UNIVERSAL SOIL LOSS EQUATION

RAINFALL FACTOR

R = 40

SOIL ERODIBILITY FACTOR

SOIL TYPE = 100% EH #24 = .16

K = .16

SLOPE FACTOR

LENGTH (ft.)	Δ ELEV (ft.)	SLOPE (%)	LS
600	30 (assumed)	5	1.31 (80)
200	50	25	8.33 (-2)
			use 2.72

COVER FACTOR

AREA (ac.)	COVER TYPE	% COVER	CANOPY (%)	WEIGHTED C
100%	disturbed	—	—	1.0

C = 1.0

EROSION CONTROL FACTOR

P = 1.0

SEDIMENT INFLOW

$A = 40(.16)(2.72)(1.0)(1.0) = 17.41$ ton/acre/year

$A = 17.41\left(\frac{1}{2047}\right)(15.1)(.95) = .122$ acre-feet/year

BY ___ DATE ___ CHECKED BY ___ COPY TO EO ___

Dames & Moore