

INSPECTION REPORT
Sedimentation Structure
J28-B
Kayenta Mine
Navajo County, Arizona

for
PEABODY COAL COMPANY



Dames & Moore
10139-011-22

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INTRODUCTION

Sedimentation Structure J28-B is an earthen embankment, designed and constructed in 1983 by Peabody Coal Company as a temporary sedimentation structure to control runoff and sediment from the disturbed mining areas of the Kayenta Mine. The location of Structure J28-B is shown on Plate 1, Site Plan.

This inspection report contains information specific to Structure J28-B. Regional site information is presented in the "General Report, Kayenta and Black Mesa Mines, Navajo County, Arizona for Peabody Coal Company," along with the methods and results of analyses used for slope stability, hydrology and hydraulics.

INSPECTION

Structure J28-B was inspected on September 12, 1985 by an interdisciplinary team of engineers from Dames & Moore. The purpose of the inspection was to assess the safety and general condition of the structure with respect to United States Department of Interior, Office of Surface Mining (OSM) regulations.

Dames & Moore's inspection was performed in accordance with applicable 30 CFR 780 and 816 regulations and included a review of the J28-B project files and a field inspection of the structure. The most current information contained in the Peabody Coal Company files includes the 1984 and current survey data and inspections performed in 1984 and 1985 by

Peabody Coal Company. The survey data developed in August 1984 was used in the analyses of the structure. Results of the field inspection are included in this report as Appendix A.

SITE DESCRIPTION

LAND USE

Structure J28-B has a 14.0-acre tributary drainage area and is located near Moenkopi Wash at the Kayenta Mine. The watershed is classified as 55% disturbed and 45% Pinion/Juniper.

EMBANKMENT

Structure J28-B is a homogeneous earthen embankment classified as a sidehill embankment. Physical characteristics of the embankment are listed in the following table:

Structure J28-B

| | |
|----------------------------|----------------------|
| Embankment | Residual Shale Soils |
| Foundation | Residual Shale Soils |
| Right Abutment | Residual Shale Soils |
| Left Abutment | Residual Shale Soils |
| Height | 19.4 ft |
| Crest Width | 13 ft |
| Upstream Slope | 2.1 H : 1 V |
| Downstream Slope | 3.1 H : 1 V |

A cross-section of the embankment is shown on Plate 2, Existing Maximum Cross Section J28-B, A-A'. Grass provides erosion protection on the downstream slope of the embankment.

ANALYSES

STABILITY

Structure J28-B is a category B-1 embankment. A standard category B-1 embankment has static and seismic factors of safety of 1.5 and 1.2, respectively, under the following conditions:

1. Maximum height = 30 ft
2. Maximum upstream slope = 2.0 H : 1 V
3. Maximum downstream slope = 2.5 H : 1 V
4. Normal pool with steady seepage saturation conditions

The J28-B embankment is lower in height and has flatter slopes than the category standard; therefore, the embankment has factors of safety greater than the design minimum.

HYDROLOGY

The hydrologic analysis was completed using the U.S. Army Corps of Engineers generalized computer program HEC-1, Flood Hydrograph Package. Structure J28-B is not in series with any other structure and therefore the spillway was analyzed using the 25-year, 6-hour storm. The storage capacity of Structure J28-B was analyzed using the 10-year, 24-hour storm.

The following parameters were used in the hydrologic analysis:

1. Water Course length, L 0.152 mi
2. Elevation Difference, H 47 ft
3. Time of Concentration, T_c 0.067 h
4. Lag time, $0.6T_c$ 0.040 h
5. SCS Curve Number 89
6. Rainfall Depth, 10-year, 24-hour storm . 2.1 in.
25-year, 6-hour storm. . 1.9 in.
7. Drainage Area 14.0 acres

HYDRAULICS

The HEC-1 program was used to evaluate inflow to the sedimentation structure, outflow from the structure and the resulting water surface elevations. The initial conditions and results of the analysis are summarized in the following table.

J28-B HYDRAULICS

| | Units | 10-year 24-hour Storm | 25-year 6-hour Storm |
|--------------------------|---------|-----------------------------|--------------------------------------|
| <hr/> | | | |
| Initial Reservoir Volume | | | |
| Condition | | Empty | Full to the spillway elevation |
| Inflow | | | |
| Peak Flow | cfs | 35 | 44 |
| Volume | acre-ft | 1.45 | 1.21 |
| Storage | | | |
| Peak Stage | ft | 6842.51 | 6858.31 |
| Spillway Elevation . . | ft | 6857.90 | — |
| Peak Storage | acre-ft | 1.45 | — |
| Storage Capacity . . . | acre-ft | 19.8 | — |
| Outflow | | | |
| Peak Flow | cfs | 0 | 2 |
| Embankment Crest | | | |
| Elevation | ft | — | 6860.50 |
| Peak Stage | ft | — | 6858.30 |
| Freeboard | ft | — | 2.20 |
| Spillway Channel | | | |
| Flow Depth | ft | — | 0.40 |
| Critical Velocity . . . | fps | — | 1.6 |
| Manning's "n" | | — | 0.035 |
| Outflow Channel | | | |
| | | | <u>Section I</u> <u>Section II</u> |
| Slope | % | — | 18 5 |
| Normal Velocity | fps | — | 2.4 1.5 |
| Normal Depth | ft | — | 0.05 0.08 |
| Manning's "n" | | — | 0.035 0.035 |

Approach Channel

The existing approach channel for J28-B has the following dimensions:

| | |
|--------------------------|-------------|
| Channel width | 15 to 35 ft |
| Channel length | 40 ft |
| Average slope | 8 percent |

Spillway Channel

The existing spillway for J28-B has a trapezoidal channel with the following dimensions:

| | |
|---|-----------|
| Channel depth | 2.6 ft |
| Channel width | 15 ft |
| Channel length | 35 ft |
| Side slopes (horizontal to vertical). . | 2:1 |
| Average exit slope | 0 percent |

There is presently no erosion protection within the channel.

Outflow Channel

The existing outflow channel for J28-B has a trapezoidal shape with the following dimensions:

| | |
|---|-----------|
| Channel width | 15 ft |
| Channel length | 50 ft |
| Side slopes (horizontal to vertical). . | 2:1 |
| Average exit slope | 5 percent |

There is presently no erosion protection within the channel.

STORAGE CAPACITY

The impoundment volume-elevation curve is based on site specific surveys conducted for Peabody Coal Company's August 1984 inspection, and 1985 resurveys, where available. Additionally, the most current topographic maps available were used in developing Plate 3, Volume-Elevation Curve, J28-B.

The calculations for the sediment load entering Structure J28-B were made utilizing the Universal Soil Loss Equation with the following parameters:

1. Rainfall Factor, R 40
2. Soil Erodibility Factor, K 0.32
3. Slope Factor, LS 1.54
4. Cover Factor, C 0.730
5. Erosion Control Factor, P 1.0

The hydrologic analysis gives the storage volume required to contain the 10-year, 24-hour storm, and the remaining storage volume available for storing sediment. The existing storage capacity of J28-B is shown on Plate 3, Volume-Elevation Curve, J28-B, and the results of the analysis are summarized in the following table.

J28-B STORAGE

| | | |
|---|-------|------------|
| Total Storage Capacity | 19.8 | acre-ft |
| 10-year, 24-hour Storm Inflow | 1.45 | acre-ft |
| Available Sediment Storage Capacity . . | 18.35 | acre-ft |
| Sediment Inflow Rate | 0.094 | acre-ft/yr |
| Sediment Storage Life | 195 | yrs |

REMEDIAL COMPLIANCE PLAN

GEOTECHNICS

The inspection of Structure J28-B indicated that the only geotechnical problem is rill erosion on the upstream slope. Correction of erosion is considered a periodic maintenance task and does not require remedial action.

HYDRAULICS

The storage capacity and spillway capacity of Structure J28-B are adequate; however, the spillway does not have an adequate outflow channel or adequate erosion protection. A trapezoidal outflow channel should be constructed along the alignment B-B' shown in Plate 1. The channel profile is shown in Plate 4 and the required dimensions are shown in Plate 5. Both the spillway and outflow channel should be protected against erosion using geotextile and gravel as shown in Plate 5.

* * *

The following plates and appendix are attached and complete this inspection report.

Plate 1 - Site Plan J28-B

Plate 2 - Existing Maximum Cross Section J28-B, A-A'

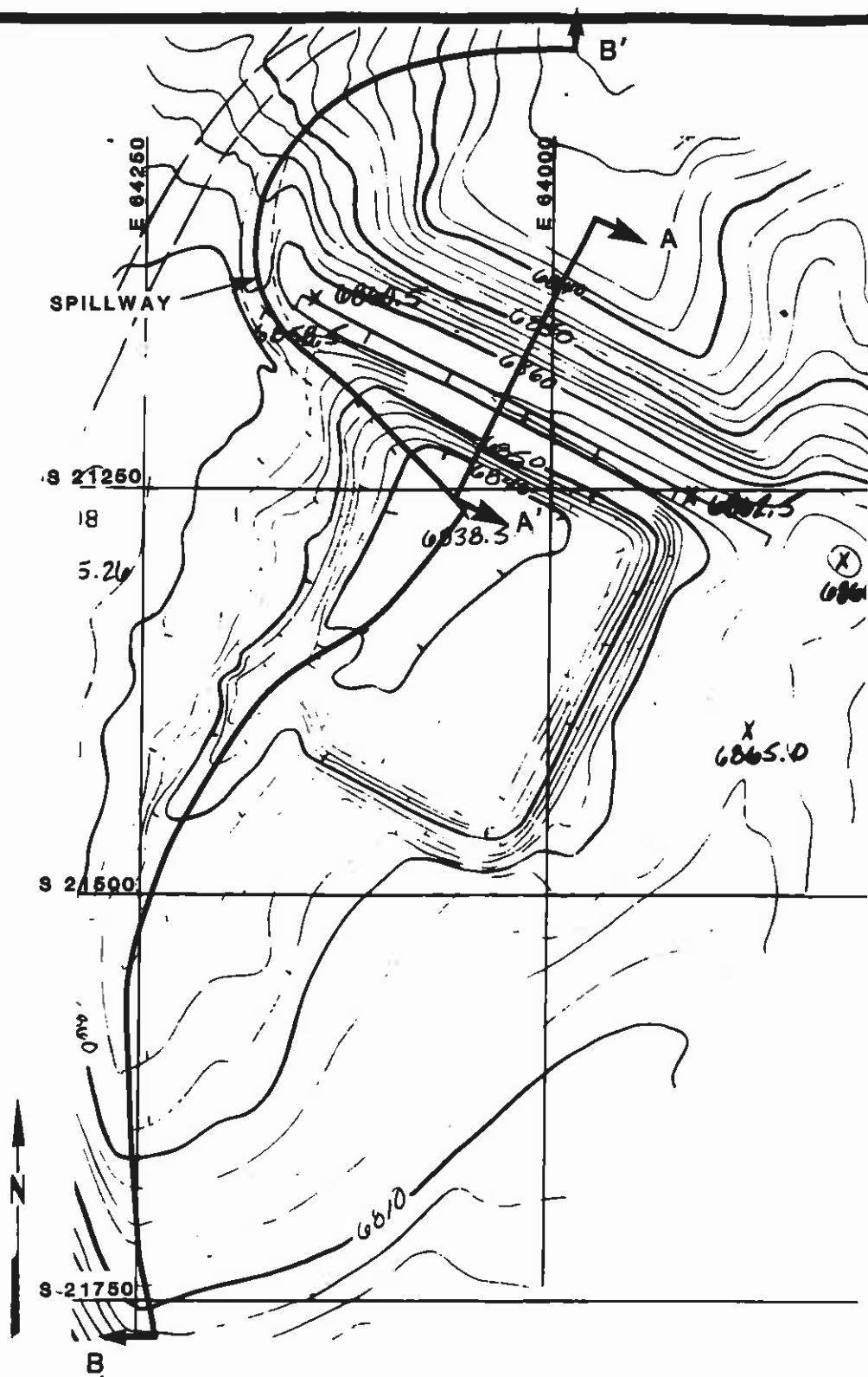
Plate 3 - Volume-Elevation Curve J28-B

Plate 4 - Channel Profile J28-B, B-B'

Plate 5 - Spillway and Outflow Channel Cross Section J28-B

Appendix A - Inspection Check List

Appendix B - Hydrology and Hydraulic Calculations

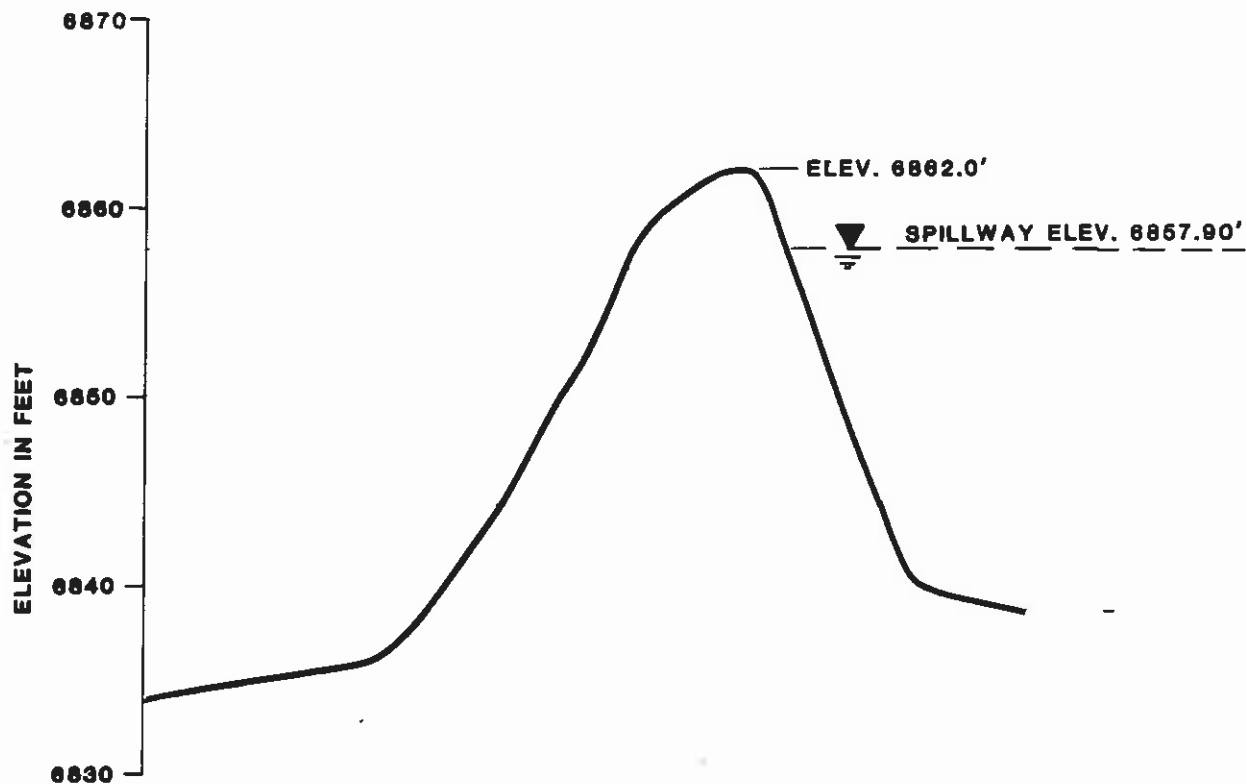


**SITE PLAN
J28-B**

SCALE
0 100 200
FEET

BY **Dames & Moore**

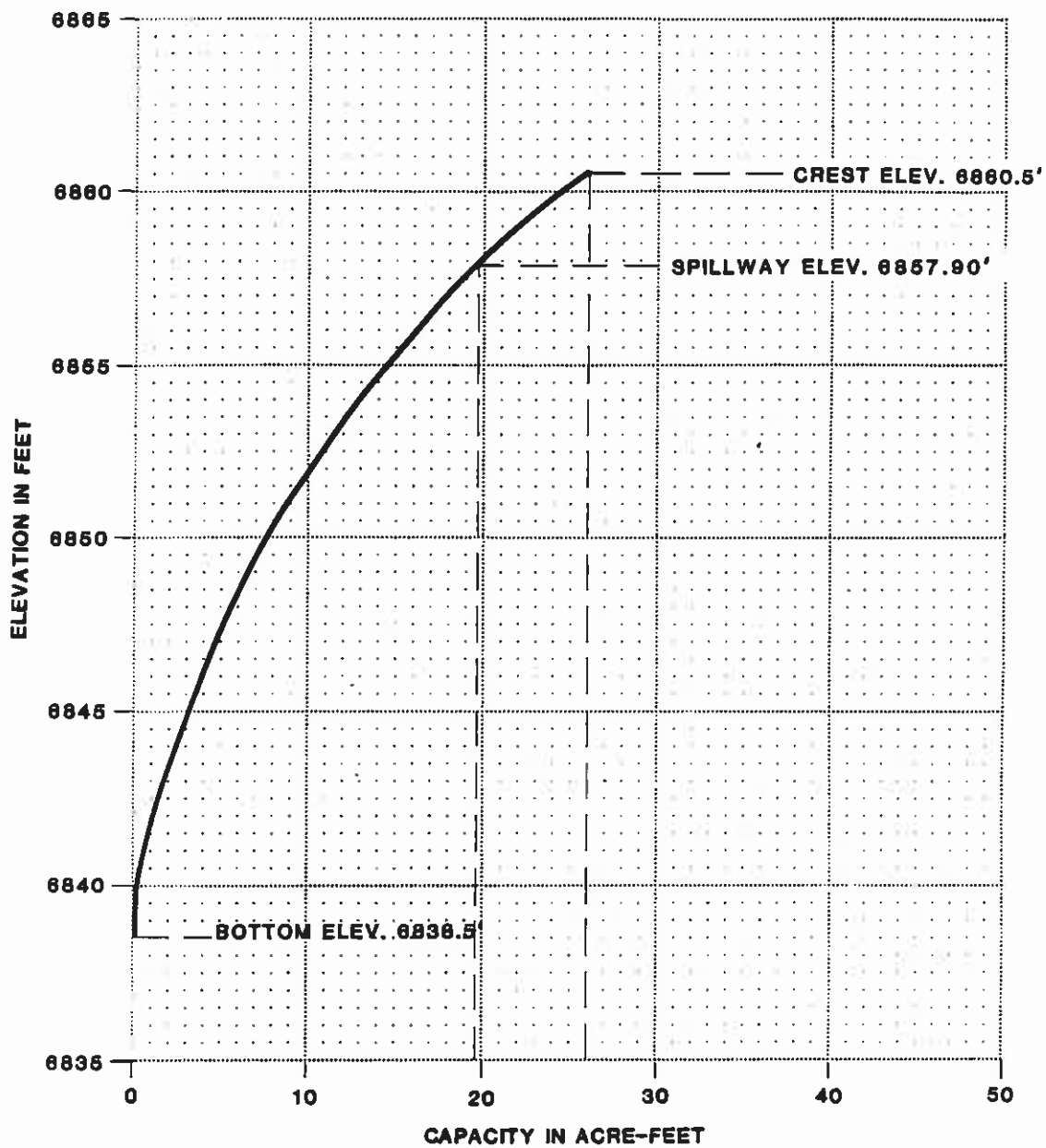
Plate 1



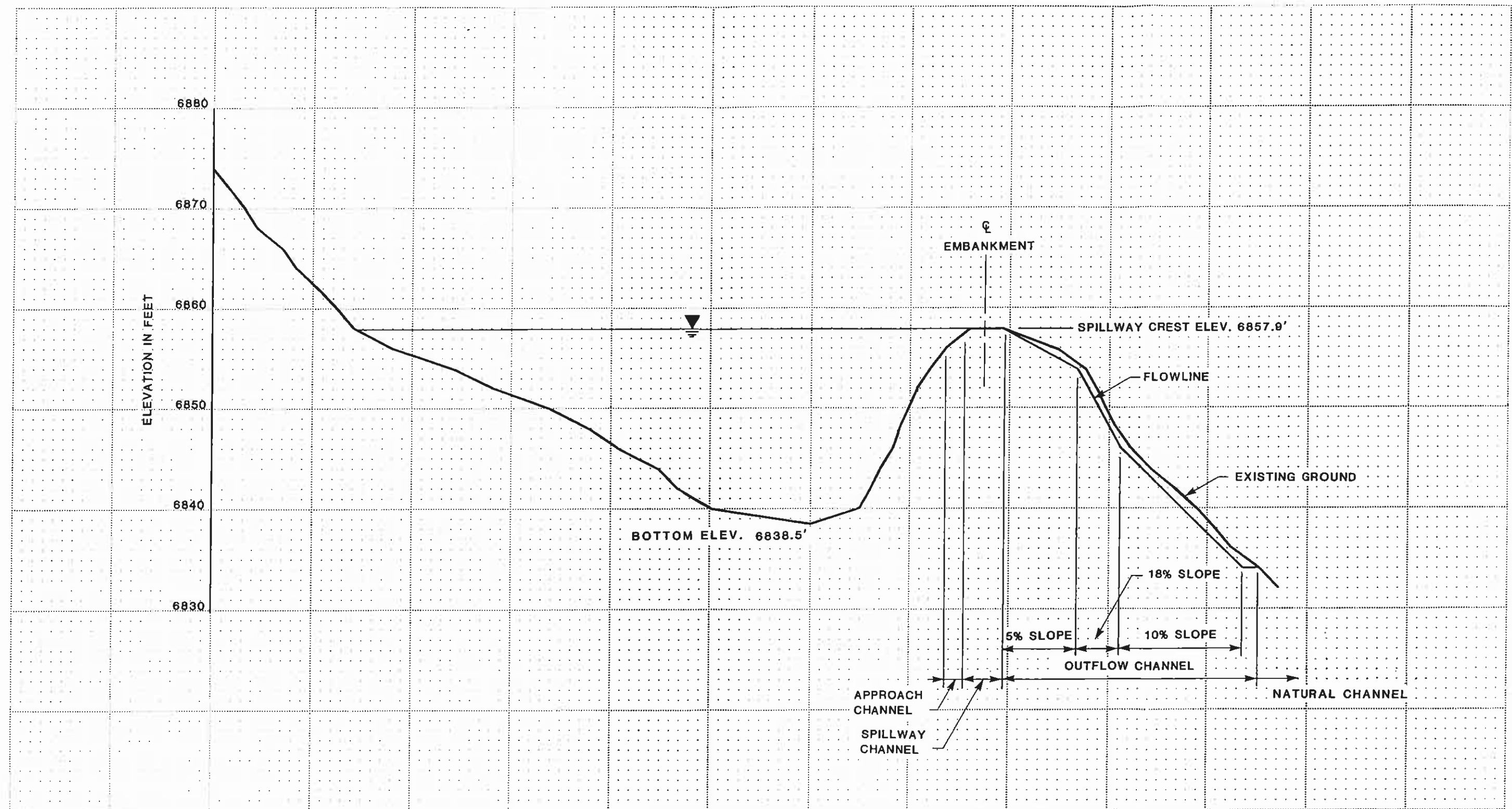
EXISTING
MAXIMUM CROSS-SECTION
A-A'
J28-B

BY **Dames & Moore**

Plate 2

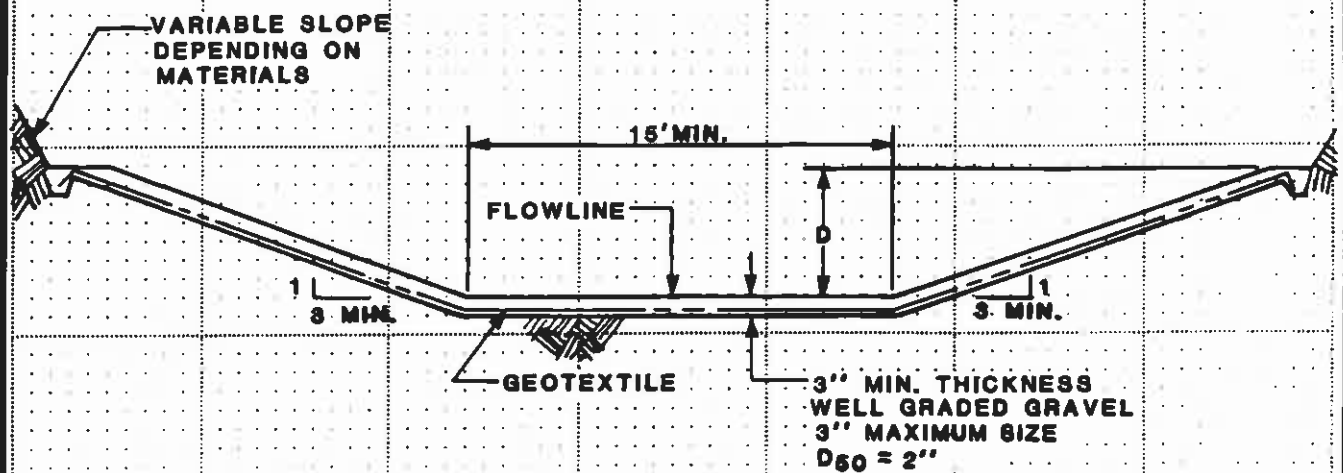


**VOLUME-ELEVATION
CURVE
J28-B**



CHANNEL PROFILE B-B'
J28-B

SCALE
0 100 200
FEET



SPILLWAY CHANNEL

$D = 1.4'$

LENGTH = 40'

FLOWLINE ELEV. = 6857.90'

OUTFLOW CHANNEL

$D = 1'$

**SPILLWAY AND
OUTFLOW CHANNEL
CROSS SECTION
J28-B**

APPENDIX A
INSPECTION CHECK LIST

Sideline

INSPECTION CHECK LIST

| ITEM | YES | NO | REMARKS |
|----------------------------------|-----|----|--------------------------------------|
| 1. CREST | | | 13' w |
| a. Any visual settlements? | | X | |
| b. Misalignment? | | X | |
| c. Cracking? | | X | |
| 2. UPSTREAM SLOPE | | | 26 1/2° |
| a. Adequate grass cover? | | X | |
| b. Any erosion? | X | | Rills |
| c. Are trees growing on slope? | | X | |
| d. Longitudinal cracks? | | X | |
| e. Transverse cracks? | | X | |
| f. Adequate riprap protection? | | X | |
| g. Any stone deterioration? | | | NA |
| h. Visual depressions or bulges? | | X | |
| i. Visual settlements? | | X | |
| j. Animal burrows? | | X | |
| 3. DOWNSTREAM SLOPE | | | 18° |
| a. Adequate grass cover? | X | | |
| b. Any erosion? | | X | |
| c. Are trees growing on slope? | | X | |
| d. Longitudinal cracks? | | X | |
| e. Transverse cracks? | | X | |
| f. Visual depressions or bulges? | | X | |
| g. Visual settlements? | | X | |
| h. Is the toe drain dry? | | | NA |
| i. Are the relief wells flowing? | | | NA |
| j. Are boils present at the toe? | | X | |
| k. Is seepage present? | | X | |
| l. Animal burrows? | | X | |
| 4. ABUTMENT CONTACT. RIGHT | | | Some erosion into pond away from dam |
| a. Any erosion? | | X | |
| b. Visual differential movement? | | X | |
| c. Any cracks noted? | | X | |
| d. Is seepage present? | | X | |
| e. Type of Material? | | | SM brown |
| 5. ABUTMENT CONTACT. LEFT | | | |
| a. Any erosion? | | X | |
| b. Visual differential movement? | | X | |
| c. Any cracks noted? | | X | |
| d. Is seepage present? | | X | |
| e. Type of Material? | | | brown SM |

| ITEM | YES | NO | REMARKS |
|------------------------------|-----|----|------------------------------------|
| 6. SPILLWAY/NORMAL | | | |
| a. Location: | | | |
| Left abutment? | X | | |
| Right abutment? | | | |
| Crest of Embankments? | | | |
| b. Approach Channel: | X | | 15' W at gateway 35' L at entrance |
| Are side slopes eroding? | | X | 8% slope |
| Are side slopes sloughing? | | X | 3.1' below crest |
| Bottom of channel eroding? | | X | |
| Obstructed? | | X | |
| Erosion protection? | | X | |
| c. Spillway Channel: | X | | 15' W 35' L 0% slope |
| Are side slopes eroding? | | X | |
| Are side slopes sloughing? | | X | |
| Bottom of channel eroding? | | X | |
| Obstructed? | | X | |
| Erosion protection? | | X | |
| d. Outflow Channel: | X | | 15' W 50' L 5% SL |
| Are side slopes eroding? | | X | |
| Are side slopes sloughing? | | X | |
| Bottom of channel eroding? | | X | |
| Obstructed? | | X | |
| Erosion protection? | | X | |
| e. Weir: | | X | |
| Condition? | | | |
| 7. SPILLWAY/EMERGENCY | | | NA |
| a. Location: | | | |
| Left abutment? | | | |
| Right abutment? | | | |
| Crest of Embankments? | | | |
| b. Approach Channel: | | | |
| Are side slopes eroding? | | | |
| Are side slopes sloughing? | | | |
| Bottom of channel eroding? | | | |
| Obstructed? | | | |
| Erosion protection? | | | |
| c. Spillway Channel: | | | |
| Are side slopes eroding? | | | |
| Are side slopes sloughing? | | | |
| Bottom of channel eroding? | | | |
| Obstructed? | | | |
| Erosion protection? | | | |
| d. Outflow Channel: | | | |
| Are side slopes eroding? | | | |
| Are side slopes sloughing? | | | |
| Bottom of channel eroding? | | | |
| Obstructed? | | | |
| Erosion protection? | | | |
| e. Weir: | | | |
| Condition? | | | |

| ITEM | YES | NO | REMARKS |
|--------------------------------|-----|-----------|---------|
| 8. IMPOUNDMENT | | | |
| a. Sinkholes? | | X (Elev.) | feet |
| b. Water present? | X | (Elev.) | feet |
| c. Siltation? | X | | |
| d. Watershed matches soil map? | | X | |

9. GENERAL COMMENTS

| |
|--|
| |
| |
| |
| |
| |
| |
| |
| |
| |
| |

Canopy Cover 0

Ground Cover 25

APPENDIX B
HYDROLOGY AND HYDRAULIC CALCULATIONS

TIME OF CONCENTRATION

ELEVATION DIFFERENCE = 6905 - 6858 = 47 ft.

WATER COURSE LENGTH = 2.0 (400) = 800 ft. = 0.152 mi.

$$T_c = \left(\frac{11.9 (0.152)^3}{47} \right)^{0.385} = 0.067 \text{ hr.}$$

Lag Time = 2.6 T_c = 0.040 hr.

SCS CURVE NUMBER

| DRAINAGE AREA (ac) | COVER TYPE | HYDROLOGIC CONDITION | SOIL TYPE | WEIGHTED CURVE NUMBER |
|-----------------------|---------------|-------------------------|--------------|--------------------------|
| 6.3 | P-J | poor | C | 85(.45) |
| 7.7 | disturbed | — | C | 91(.55) |
| | | | | <u>88.3</u> |

100% E4 # 26

use 89

DRAINAGE BASIN AREA

14.0 ACRES

0.022 SQ MILES

REVISIONS

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BY S. DOLAN DATE 10-2-85

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UNIVERSAL SOIL LOSS EQUATION

RAINFALL FACTOR

$$R = 40$$

SOIL ERODIBILITY FACTOR

$$\text{SOIL TYPE} = 100\% \text{ EH \#26} = .32$$

$$K = \underline{\underline{.32}}$$

SLOPE FACTOR

| <u>LENGTH (ft.)</u> | <u>Δ ELEV (ft.)</u> | <u>SLOPE (%)</u> | <u>LS</u> |
|---------------------|---------------------|------------------|----------------------|
| 250 | 20 | 8 | 1.57 (.5) |
| 500 | 30 | 6 | 1.51 (.5) |
| | | | <u><u>= 1.54</u></u> |

COVER FACTOR

| <u>AREA (ac.)</u> | <u>COVER TYPE</u> | <u>% COVER</u> | <u>CANOPY (%)</u> | <u>WEIGHTED C</u> |
|-------------------|-------------------|----------------|-------------------|-----------------------|
| 45% | P-J | 10 | 25 | .45 (.40) |
| 55% | disturbed | — | — | .55 (1.0) |
| | | | | <u><u>C = .73</u></u> |

EROSION CONTROL FACTOR

$$P = 1.0$$

SEDIMENT INFLOW

$$A = 40 (.32) (1.54) (.73) (1.0) = 14.39 \text{ ton/acre/year}$$

$$A = (14.39) \left(\frac{1}{2047} \right) (14.0) (.95) = 0.094 \text{ acre-feet/year}$$

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